

Since confined masonry buildings performed well in past earthquakes, resources related to seismic repair and retrofit of these buildings are limited. The reader is referred to a publication developed after the 2009 Pisco, Peru earthquake (PNUD, 2009), and another one prepared after the 2002 Colima, Mexico earthquake (EERI, 2006).



a)



b)

Figure 4. Performance of confined masonry buildings in recent significant earthquakes: a) the 2010 Maule, Chile earthquake (M.O. Moroni Yadlin), and b) the 2007 Pisco, Peru earthquake (D. Quiun).

1.3.2 General System Behavior

1.3.2.1 How Seismic Forces are Resisted by a Confined Masonry Wall Panel

Seismic behavior of a confined masonry wall panel can be explained by composite (monolithic) action of a masonry wall and adjacent RC confining elements. This composite action exists due to the tootling between the walls and the tie-columns - that is one of the key features of confined masonry construction. In the absence of tootling, composite action can be achieved by means of horizontal reinforcement (dowels). Figure 5 shows a two-bay confined masonry specimen subjected to reversed cyclic lateral loading simulating earthquake effects (Pérez-Gavián, 2009). The specimen demonstrated a typical damage pattern in the form of diagonal shear cracks. The failure took place in the form of a single diagonal crack which propagated through the walls and the tie-columns. This mechanism can be expected to occur in buildings with small RC tie-column sizes, where tie-column depth does not exceed 1.5 times the wall thickness.

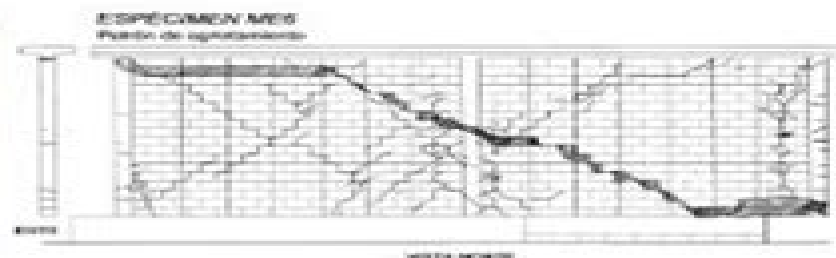


Figure 5. Failure of a two-bay confined masonry wall (Pérez-Gavián, 2009).

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