

MAX SERVICE LOADS AT LINK END

$$\begin{aligned} V_{oc} &= 1 \text{ kips} \\ M_{oc} &= 1 \text{ ft-kips} \\ V_{uc} &= 1 \text{ kips} \\ M_{uc} &= 1 \text{ ft-kips} \end{aligned}$$

$$\begin{array}{ccccc} 20 & 10.4 & 0.47 & 10.13 & 0.77 & 75.7 \\ I_x & r_x & r_y & Z_x & k & \\ 394 & 4.44 & 2.59 & 85 & 1.38 & \end{array}$$

MAX HORIZ. SEISMIC LOADS AT LINK END

$$\begin{aligned} V_g &= 68.95 \text{ kips (CBC 30A-1)} \\ P_g &= 56 \text{ kips (CBC 30A-1)} \\ M_g &= 139 \text{ ft-kips (CBC 30A-1)} \end{aligned}$$

THE LINK DESIGN IS ADEQUATE.
(USE 3/8 x 4-5/8 @ 24 in o.c. INTERMEDIATE & END STIFFENERS WITH 1/4" FILLET WELD.)

LINK LENGTH

$$e = 4 \text{ ft}$$

LINK YIELD STRESS

$$F_y = 50 \text{ ksi}$$

REDUNDANCY FACTOR

$$\rho = 1.5$$

IMPORTANCE FACTOR

$$I = 1.15 \text{ (CBC Tab. 16A-K)}$$

SEISMIC COEFFICIENT

$$C_s = 0.44 \text{ (CBC Tab. 16A-Q)}$$

BEAM LENGTH BETWEEN COL. CENTERS

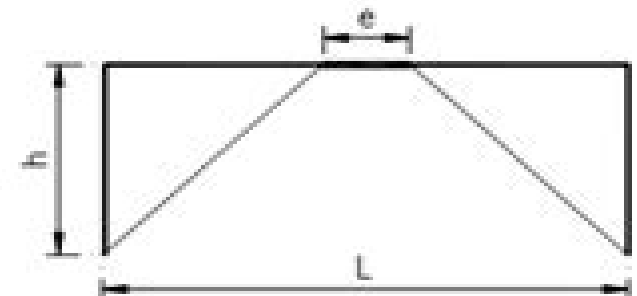
$$L = 28 \text{ ft (including link)}$$

STORY HEIGHT

$$h = 14 \text{ ft}$$

MAXIMUM INELASTIC STORY DRIFT

$$\Delta_u = 0.65 \text{ in (CBC eq 30A-17)}$$



THE ALLOWABLE DESIGN LOADS (CBC Sec. 1812A.3.1)

$$\begin{aligned} LC1 &= D + 0.75[L + (\rho E_h + 0.5C_s D) / 1.4] \\ LC2 &= D + (\rho E_h + 0.5C_s D) / 1.4 \\ V &= \text{MAX}(LC1, LC2) = 75.1 \text{ kips} \\ P &= \text{MAX}(LC1, LC2) = 60.0 \text{ kips} \\ M &= \text{MAX}(LC1, LC2) = 150.1 \text{ ft-kips} \end{aligned}$$

THE LOCAL BUCKLING LIMITATION (CBC Sec. 2213A.10.2, ASD Tab. B5.1)

$$\begin{aligned} b_f / (2t_f) &= 6.58 < 52 / (F_y)^{0.4} = 7.35 \text{ [Satisfactory]} \\ d / t_w &= 22.13 < \begin{cases} 640 (1 - 3.74 P_c / P_y) / (F_y)^{0.4} = 70.20 \text{ , for } P_c / P_y < 0.16 \\ 257 / (F_y)^{0.4} = \text{N/A} \text{ , for } P_c / P_y > 0.16 \end{cases} \\ &\text{[Satisfactory]} \end{aligned}$$

$$\text{Where } P_y = F_y A = 1000 \text{ kips}$$

THE FLEXURAL CAPACITY (CBC 2213A.10.3)

$$\begin{aligned} M_{rs} &= Z_x (f_y - f_a) = 328.1 \text{ ft-kips} > M \text{ [Satisfactory]} \\ \text{Where } f_a &= P / (2b_f t_f) = 3.85 \text{ ksi} \\ A_f &= b_f t_f = 7.80 \text{ in}^2 \end{aligned}$$

THE SHEAR CAPACITY (CBC 2213A.10.3, 2213A.10.5)

$$\Omega = V_g / V = 1.79 > \Omega_{min} = 1.0 / 0.8 = 1.25 \text{ [Satisfactory]}$$

Seismic Engineering 2001

William Ingersoll Rose



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