

MAX SERVICE LOADS AT LINK END

V_{oc}	=	1	kips
M_{oc}	=	1	ft-kips
V_{LL}	=	1	kips
M_{LL}	=	1	ft-kips

20	10.4	0.47	10.13	0.77	75.7
	I_x	r_x	r_y	Z_x	k
	394	4.44	2.59	85	1.38

MAX HORIZ. SEISMIC LOADS AT LINK END

V_g	=	68.95	kips (CBC 30A-1)
P_g	=	56	kips (CBC 30A-1)
M_g	=	139	ft-kips (CBC 30A-1)

THE LINK DESIGN IS ADEQUATE.
(USE 3/8 x 4-5/8 @ 24 in o.c. INTERMEDIATE & END STIFFENERS WITH 1/4" FILLET WELD.)

LINK LENGTH

e	=	4	ft
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LINK YIELD STRESS

F_y	=	50	ksi
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REDUNDANCY FACTOR

ρ	=	1.5
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IMPORTANCE FACTOR

I	=	1.15	(CBC Tab. 16A-K)
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SEISMIC COEFFICIENT

C_g	=	0.44	(CBC Tab. 16A-Q)
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BEAM LENGTH BETWEEN COL. CENTERS

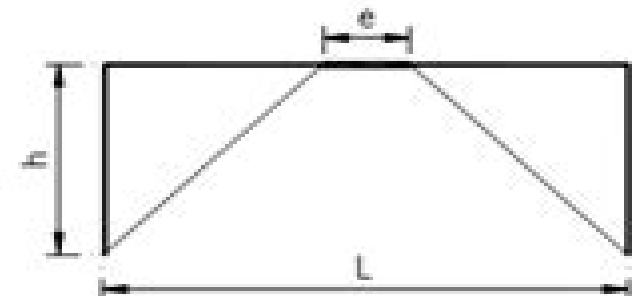
L	=	28	ft (including link)
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STORY HEIGHT

h	=	14	ft
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MAXIMUM INELASTIC STORY DRIFT

Δ_u	=	0.65	in (CBC eq 30A-17)
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THE ALLOWABLE DESIGN LOADS (CBC Sec. 1812A.3.1)

$$LC1 = D + 0.75[L + (\rho E_b + 0.5C_g D) / 1.4]$$

$$LC2 = D + (\rho E_b + 0.5C_g D) / 1.4$$

$$V = \text{MAX}(LC1, LC2) = 75.1 \text{ kips}$$

$$P = \text{MAX}(LC1, LC2) = 60.0 \text{ kips}$$

$$M = \text{MAX}(LC1, LC2) = 150.1 \text{ ft-kips}$$

THE LOCAL BUCKLING LIMITATION (CBC Sec. 2213A.10.2, ASD Tab. B5.1)

$$b_f / (2t_f) = 6.58 < 52 / (F_y)^{0.4} = 7.35 \quad \text{[Satisfactory]}$$

$$d / t_w = 22.13 < \begin{cases} 640 (1 - 3.74 P_c / P_y) / (F_y)^{0.4} = 70.20 & \text{for } P_c / P_y < 0.16 \\ 257 / (F_y)^{0.4} = \text{N/A} & \text{for } P_c / P_y > 0.16 \end{cases}$$

[Satisfactory]

$$\text{Where } P_y = F_y A = 1000 \text{ kips}$$

THE FLEXURAL CAPACITY (CBC 2213A.10.3)

$$M_{rs} = Z_x (f_y - f_a) = 328.1 \text{ ft-kips} > M \quad \text{[Satisfactory]}$$

$$\text{Where } f_a = P / (2b_f t_f) = 3.85 \text{ ksi}$$

$$A_f = b_f t_f = 7.80 \text{ in}^2$$

THE SHEAR CAPACITY (CBC 2213A.10.3, 2213A.10.5)

$$\Omega = V_g / V = 1.79 > \Omega_{min} = 1.0 / 0.8 = 1.25 \quad \text{[Satisfactory]}$$

Seismic Engineering 2001

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