

MAX SERVICE LOADS AT LINK END

$$\begin{aligned} V_{oc} &= 1 \text{ kips} \\ M_{oc} &= 1 \text{ ft-kips} \\ V_{uc} &= 1 \text{ kips} \\ M_{uc} &= 1 \text{ ft-kips} \end{aligned}$$

$$\begin{array}{ccccc} 20 & 10.4 & 0.47 & 10.13 & 0.77 & 75.7 \\ I_x & r_x & r_y & Z_x & k & \\ 394 & 4.44 & 2.59 & 85 & 1.38 & \end{array}$$

MAX HORIZ. SEISMIC LOADS AT LINK END

$$\begin{aligned} V_g &= 68.95 \text{ kips (CBC 30A-1)} \\ P_g &= 56 \text{ kips (CBC 30A-1)} \\ M_g &= 139 \text{ ft-kips (CBC 30A-1)} \end{aligned}$$

THE LINK DESIGN IS ADEQUATE.
(USE 3/8 x 4-5/8 @ 24 in o.c. INTERMEDIATE & END STIFFENERS WITH 1/4" FILLET WELD.)

LINK LENGTH

$$e = 4 \text{ ft}$$

LINK YIELD STRESS

$$F_y = 50 \text{ ksi}$$

REDUNDANCY FACTOR

$$\rho = 1.5$$

IMPORTANCE FACTOR

$$I = 1.15 \text{ (CBC Tab. 16A-K)}$$

SEISMIC COEFFICIENT

$$C_s = 0.44 \text{ (CBC Tab. 16A-Q)}$$

BEAM LENGTH BETWEEN COL. CENTERS

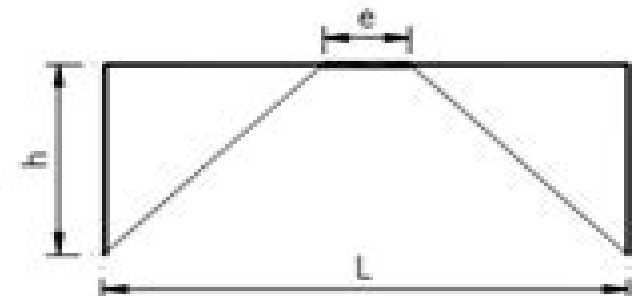
$$L = 28 \text{ ft (including link)}$$

STORY HEIGHT

$$h = 14 \text{ ft}$$

MAXIMUM INELASTIC STORY DRIFT

$$\Delta_u = 0.65 \text{ in (CBC eq 30A-17)}$$



THE ALLOWABLE DESIGN LOADS (CBC Sec. 1812A.3.1)

$$LC1 = D + 0.75[L + (\rho E_b + 0.5C_b D) / 1.4]$$

$$LC2 = D + (\rho E_b + 0.5C_b D) / 1.4$$

$$V = \text{MAX}(LC1, LC2) = 75.1 \text{ kips}$$

$$P = \text{MAX}(LC1, LC2) = 60.0 \text{ kips}$$

$$M = \text{MAX}(LC1, LC2) = 150.1 \text{ ft-kips}$$

THE LOCAL BUCKLING LIMITATION (CBC Sec. 2213A.10.2, ASD Tab. B5.1)

$$b_f / (2t_f) = 6.58 < 52 / (F_y)^{0.4} = 7.35 \text{ [Satisfactory]}$$

$$d / t_w = 22.13 < \begin{cases} 640 (1 - 3.74 P_c / P_y) / (F_y)^{0.4} = 70.20 & \text{for } P_c / P_y < 0.16 \\ 257 / (F_y)^{0.4} = \text{N/A} & \text{for } P_c / P_y > 0.16 \end{cases}$$

[Satisfactory]

$$\text{Where } P_y = F_y A = 1000 \text{ kips}$$

THE FLEXURAL CAPACITY (CBC 2213A.10.3)

$$M_{tr} = Z_x (f_y - f_a) = 328.1 \text{ ft-kips} > M \text{ [Satisfactory]}$$

$$\text{Where } f_a = P / (2b_f t_f) = 3.85 \text{ ksi}$$

$$A_f = b_f t_f = 7.80 \text{ in}^2$$

THE SHEAR CAPACITY (CBC 2213A.10.3, 2213A.10.5)

$$\Omega = V_g / V = 1.79 > \Omega_{min} = 1.0 / 0.8 = 1.25 \text{ [Satisfactory]}$$

Seismic Engineering 2001

Yousef Bozorgnia, Vitelmo V. Bertero



Seismic Engineering 2001:

Seismic Engineering-2001, 2001 The Nisqually, Washington, Earthquake of February 28, 2001 : Lifeline Performance
Peter W. McDonough, 2002-01-01 Assesses damage to buildings and infrastructure as a result of the February 2001 earthquake near Olympia that also impacted Seattle The 12 chapters evaluate the performance of the highway systems power systems water supply railways gas and oil pipelines communication facilities airports mari *Earthquake Engineering* Yousef Bozorgnia, Vitelmo V. Bertero, 2004-05-11 This multi contributor book provides comprehensive coverage of earthquake engineering problems an overview of traditional methods and the scientific background on recent developments It discusses computer methods on structural analysis and provides access to the recent design methodologies and serves as a reference for both professionals and res *2nd fib Congress in Naples Italy Vol1* FIB - International Federation for Structural Concrete, 2006-06-01 **Hazard Analysis of Seismic Soil Liquefaction** Yu Huang, Miao Yu, 2017-04-11 This book presents comprehensive hazard analysis methods for seismic soil liquefaction providing an update on soil liquefaction by systematically reviewing the phenomenon s occurrence since the beginning of this century It also puts forward a range of advanced research methods including in situ tests laboratory studies physical model tests numerical simulation and performance based assessment Recent seismic liquefaction related damage to soils and foundations demonstrate the increasing need for the comprehensive hazard analysis of seismic soil liquefaction in order to mitigate this damage and protect human lives As such the book addresses the comprehensive hazard analysis of seismic soil liquefaction including factors such as macroscopic characteristics evaluating the liquefaction potential dynamic characteristics and deformation processes providing reliable evaluation results for liquefaction potential and deformation in the context of risk assessment p **Retrofitting of Concrete Structures by Externally Bonded FRPs, With Emphasis on Seismic Applications** fib Fédération internationale du béton, 2006-01-01 fib Bulletin 35 is the first bulletin to publish documentation from an fib short course These courses are held worldwide and cover advanced knowledge of structural concrete in general or specific topics They are organized by fib and given by internationally recognized experts in fib often supplemented with local experts active in fib They are based on the knowledge and expertise from fib s ten Commissions and nearly fifty Task Groups fib Bulletin 35 presents the course materials developed for the short course Retrofitting of Concrete Structures through Externally Bonded FRP with emphasis on Seismic Applications given in Ankara and Istanbul in June 2005 The course drew on expertise both from outside Turkey and from the large pool of local experts on this subject In most countries of the world the building stock is ageing and needs continuous maintenance or repair Moreover the majority of existing constructions are deficient in the light of current knowledge and design codes The problem of structural deficiency of existing constructions is especially acute in seismic regions as even there seismic design of structures is relatively recent The direct and indirect costs of demolition and reconstruction of structurally deficient constructions are often prohibitive

furthermore they entail a substantial waste of natural resources and energy Therefore structural retrofitting is becoming increasingly widespread throughout the world Externally bonded Fibre Reinforced Polymers FRPs are rapidly becoming the technique of choice for structural retrofitting They are cleaner and easier to apply than conventional retrofitting techniques reduce disruption to the occupancy and operation of the facility do not generate debris or waste and reduce health and accident hazards at the construction site as well as noise and air pollution in the surroundings fib Bulletin 35 gives state of the art coverage of retrofitting through FRPs and presents relevant provisions from three recent standardisation milestones EN 1998 3 2005 Eurocode 8 Design of structures for earthquake resistance Part 3 Assessment and retrofitting of buildings the 2005 Draft of the Turkish seismic design code and the Italian regulatory document CNR DT 200 04 Instructions for Design Execution and Control of Strengthening Interventions by Means of Fibre Reinforced Composites 2004 *Seismic Assessment and Retrofit of Reinforced Concrete Buildings* fib Fédération internationale du béton, 2003-08-01 In most parts of the developed world the building stock and the civil infrastructure are ageing and in constant need of maintenance repair and upgrading Moreover in the light of our current knowledge and of modern codes the majority of buildings stock and other types of structures in many parts of the world are substandard and deficient This is especially so in earthquake prone regions as even there seismic design of structures is relatively recent In those regions the major part of the seismic threat to human life and property comes from old buildings Due to the infrastructure s increasing decay frequently combined with the need for structural upgrading to meet more stringent design requirements especially against seismic loads structural retrofitting is becoming more and more important and receives today considerable emphasis throughout the world In response to this need a major part of the fib Model Code 2005 currently under development is being devoted to structural conservation and maintenance More importantly in recognition of the importance of the seismic threat arising from existing substandard buildings the first standards for structural upgrading to be promoted by the international engineering community and by regulatory authorities alike are for seismic rehabilitation of buildings This is the case for example of Part 3 Strengthening and Repair of Buildings of Eurocode 8 i e of the draft European Standard for earthquake resistant design and which is the only one among the current 2003 set of 58 Eurocodes attempting to address the problem of structural upgrading It is also the case of the recent 2001 ASCE draft standard on Seismic evaluation of existing buildings and of the 1996 Law for promotion of seismic strengthening of existing reinforced concrete structures in Japan As noted in Chapter 1 of this Bulletin fib as CEB and FIP did before has placed considerable emphasis on assessment and rehabilitation of existing structures The present Bulletin is a culmination of this effort in the special but very important field of seismic assessment and rehabilitation It has been elaborated over a period of 4 years by Task Group 7 1 Assessment and retrofit of existing structures of fib Commission 7 Seismic design a truly international team of experts representing the expertise and experience of all the important seismic regions of the world In the course of its work the team had six plenary two day meetings in January 1999 in Pavia Italy in

August 1999 in Raleigh North Carolina in February 2000 in Queenstown New Zealand in July 2000 in Patras Greece in March 2001 in Lausanne Switzerland and in August 2001 in Seattle Washington In October 2002 the final draft of the Bulletin was presented to public during the 1st fib Congress in Osaka It was also there that it was approved by fib Commission 7 Seismic Design The contents is structured into main chapters as follows 1 Introduction 2 Performance objectives and system considerations 3 Review of seismic assessment procedures 4 Strength and deformation capacity of non seismically detailed components 5 Seismic retrofitting techniques 6 Probabilistic concepts and methods 7 Case studies

Seismic Behavior of Steel Storage Pallet Racking Systems Carlo Andrea Castiglioni, 2016-02-04 This book presents the main outcomes of the first European research project on the seismic behavior of adjustable steel storage pallet racking systems In particular it describes a comprehensive and unique set of full scale tests designed to assess such behavior The tests performed include cyclic tests of full scale rack components namely beam to upright connections and column base connections static and dynamic tests to assess the friction factor between pallets and rack beams full scale pushover and pseudodynamic tests of storage racks in down aisle and cross aisle directions and full scale dynamic tests on two bay three level rack models The implications of the findings of this extensive testing regime on the seismic behavior of racking systems are discussed in detail highlighting e g the confirmation that under severe dynamic conditions sliding is the main factor influencing rack response This work was conceived during the development of the SEISRACKS project Its outcomes will contribute significantly to increasing our knowledge of the structural behavior of racks under earthquake conditions and should inform future rack design

Perspectives on Earthquake Geotechnical Engineering Atilla Ansal, Mohamed Sakr, 2015-04-15 This book offers a broad perspective on important topics in earthquake geotechnical engineering and gives specialists and those that are involved with research and application a more comprehensive understanding about the various topics Consisting of eighteen chapters written by authors from the most seismic active regions of the world such as USA Japan Canada Chile Italy Greece Portugal Taiwan and Turkey the book reflects different views concerning how to assess and minimize earthquake damage The authors a prominent group of specialists in the field of earthquake geotechnical engineering are the invited lecturers of the International Conference on Earthquake Geotechnical Engineering from Case History to Practice in the honour of Professor Kenji Ishihara held in Istanbul Turkey during 17-19 June 2013

Journal of the House of Representatives of the United States United States. Congress. House, 2013 Some vols include supplemental journals of such proceedings of the sessions as during the time they were depending were ordered to be kept secret and respecting which the injunction of secrecy was afterwards taken off by the order of the House

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