



Fig. 4-1 Definitions of widths (b , d , and h_x) and thickness (flange or leg thickness t and web thickness t_w) for use in Table 4-1.

COLUMN BUCKLING

The most significant parameter affecting column stability is the *slenderness ratio* Kl/r , where l is the actual unbraced length of the column, in; Kl is the effective length of the column, in; and r is the radius of gyration of the column cross section, in. Column strength equations are normally written for ideal "pin-ended" columns. To make the strength equations applicable to all columns, an *effective length factor* K is used to account for the influence of end conditions on column stability.

Two methods for determining K for a column are presented in Sec. C2 of the Commentary on the AISC LRFD Specification: a *judgmental* method and an approximate *analytical* method. A discussion of the two methods follows.

EFFECTIVE LENGTH FACTOR: JUDGMENTAL METHOD

Six cases are shown in Table 4-2 for individual columns, with their corresponding K values, both theoretical and recommended. The more conservative recommendations (from the Structural Stability Research Council) reflect the fact that perfect fixity cannot be achieved in real structures.

The LRFD Specification distinguishes between columns in braced and unbraced frames. In braced frames, sideways is inhibited by diagonal bracing or shear walls. In Table 4-2, case d (the classical pin-ended column, $K = 1.0$) as well as cases a and b represent columns in braced frames; $K \leq 1.0$. AISC recommends that K for compression members in braced frames "shall be taken as unity, unless structural analysis shows that a smaller value may be used." It is common practice to assume $K = 1.0$ for columns in braced frames.

Schaums Outline Of Structural Steel Design

Byron S. Gottfried



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